#### Comparison of Existing Joint Orientation Parameters 1 and Their Effect on Rock Erodibility in Dam Spillways 2 3 Short title: Joint Orientation Parameters and Rock Erosion 4 5 6 M-H.Wisse<sup>\*1</sup>, A.Saeidi<sup>1</sup>, M.Quirion<sup>2</sup> 7 8 <sup>1</sup>Applied Sciences, Université du Québec à Chicoutimi, 555 Boul. de l'Université, Chicoutimi, 9 QC, G7H 2B1 10 <sup>2</sup>Rock Mechanics, Hydro-Québec, 75 Boul. René-Lévesque W, Montréal, QC, H2Z 1A4 11 12 13 \*Corresponding author (e-mail: marie-helene.wissel@uqac.ca) 14 15 M-H W.: 0000-0001-7764-5024, AS: 0000-0001-6954-5453, MQ: 0000-0003-2568-6339 16

## 17 Abstract

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19 Rock mass erosion in unlined spillways causes significant structural damage and necessitates 20 expensive repairs. The rock mass is made up of blocks formed by various arrangements of joint 21 sets. The volume and the protrusion of these blocks, as well as the orientation, opening and 22 roughness of the joints, are all features that can affect rock erodibility. Most of these features are 23 incorporated in parameters developed for rock mass characterization. Three joint orientation 24 parameters are compared in this article using a database containing geological and hydraulic 25 information on scoured spillways. According to the detailed methodology, data is first classified 26 according to rock quality using the GSI<sub>chart</sub> index. Then, for each GSI<sub>chart</sub> class, data is distributed 27 according to the damage level, stream power and joint orientation parameter chosen. This study 28 shows that no joint orientation parameter is currently able to accurately represent the effect of joint 29 orientation on erosion of excellent- to poor-quality rock mass. Moreover, this study shows that the 30 GSI<sub>chart</sub> index is not a rock quality index that completely evaluates rock erosion, since some 31 relevant parameters for evaluating rock erodibility are not considered.

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## 33 Keywords: Joint orientation, erosion, rock mass, hydraulic, unlined spillway

## 34 1 Introduction

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Hydroelectricity production requires the construction of dams. For safety, emergency spillways 36 37 are built as part of these dams to carry away excess water. These spillways are excavated into the 38 bedrock, which exposes the rock mass to the erosive power of water flow. In some cases, rock 39 mass erosion in unlined spillways has been shown to cause damage to the dam's structure. The 40 erosion process occurs in all rock mass qualities and is affected by rock mass features and flow 41 characteristics. In the field of rock mechanics, several methods for evaluating rock mass erodibility 42 have been developed, such as those of Kirsten (1982), Van Schalkwyk (1994), Annandale (1995) 43 Kirsten et al. (2000), Bollaert (2004; 2010; 2002) and Pells (2016). These methods fall into two 44 main categories: semi-analytical and semi-empirical (Jalili Kashtiban et al. 2021). The main 45 existing semi-analytical method is the presented by Bollaert (2004; 2010; 2002) - the 46 Comprehensive Scour Model (CSM). Methods of Kirsten (1982), Van Schalkwyk (1994), 47 Annandale (1995), Kirsten et al. (2000) and Pells (2016) are classified as semi-empirical. Semi-48 empirical methods define a threshold where erosion in a rock mass occurs, i.e., when the stream 49 power becomes greater than a rock's resistance to erosion. Several researchers have sought to find 50 methods to define rock mass resistance to erosion according to its characteristics, such as block 51 size, joint orientation, uniaxial compressive strength, joint opening, joint roughness and joint shear 52 strength. Boumaiza et al. (2021) have developed a methodology to identify the relative impact of 53 each of these features on the hydraulic erodibility of rock. It was found that joint orientation 54 influences rock mass resistance to erosion when subjected to flow power. This article focuses on 55 semi-empirical methods involving a parameter that describes the effect of joint orientation within 56 the flow. Methods involving joint orientation parameters that are compared in this paper are 57 described below.

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The Kirsten index (N) is an excavatability rock mass index that is designed to assess the ease of excavating bedrock with a bulldozer (Kirsten 1982). This index has also been used to evaluate the erodibility of the rock mass. However, this application has been criticized. Van Schalkwyk (1994), Annandale (1995), and Kirsten et al. (2000) used the Kirsten index to define a rock erodibility limit. This limit depends on the rock's resistance to erosion, determined using Kirsten's index, and

Cup: Uplift pressure coefficient dependent on joint orientation defined through experimental tests.

on the erosive capacity of the flow (Jalili Kashtiban et al. 2021). Kirsten's index is presented inequation (1).

 $66 N = M_s K_b K_d J_s (1)$ 

67 Where  $M_s$  is the uniaxial compressive strength of the rock,  $K_b$  refers to the block size,  $K_d$  refers to the shear strength and  $J_s$  is the joint orientation parameter.  $J_s$  defines a value of the rock's 68 69 vulnerability to erosion as a function of the joint spacing ratio and the joint orientation within the 70 flow. The Kirsten index was originally designed to define the ease with which a rock mass can be 71 excavated. Since its inception, there has been a lot of criticism of this method. In particular, its 72 application to erodibility situations has been questioned by several authors and some of the 73 parameters used have been shown not to provide a satisfactory estimate of what they represent 74 (Boumaiza 2019; Pells 2016). Boumaiza et al., (2019a) reveals that the estimation of orthogonal 75 joint sets when calculating  $J_s$  is inadequate and that a better approximation of the angle between 76 joint sets should be considered. Pells (2016) notes that the M<sub>s</sub> and RQD factors of Kirsten's index 77 have a much greater impact on the final value of Kirsten's index. The maximum values of M<sub>s</sub> and 78 RQD are 280 and 100, and their minimum values are 0.87 and 5, respectively. In contrast, the 79 maximum and minimum values of the J<sub>s</sub> parameter are 1.50 and 0.37. The overall impact of the J<sub>s</sub> 80 parameter on Kirsten's index is low.

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Pells (2016) proposed two new methods to assess rock mass erodibility: the Rock Mass Erodibility Index (RMEI) and the Erodibility Geological Strength Index (eGSI) (equation 2). It is in the eGSI that he introduces  $E_{doa}$  as a new parameter to evaluate the effect of joint orientation on rock mass erodibility.  $E_{doa}$  makes it possible to evaluate rock resistance to erosion according to joint orientation and joint spacing ratio. The value of  $E_{doa}$  is added to the GSI index developed by Hoek et al. (1995).

 $88 \quad eGSI = E_{doa} + GSI_{chart} \tag{2}$ 

89 Pells (2016) used Marinos and Hoek (2000) chart to calculate GSI<sub>chart</sub>.

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Reinius (1986) and Montgomery (1984) studied the effects of the orientation of blocks aligned in
a horizontal flow channel. Along with different block dispositions (such as orientation and
protrusion) and various flow rates, pressures applied on one of the blocks were measured and the
uplift pressure coefficient C<sub>up</sub> was obtained.

95 The two main parameters that evaluate the effect of joint orientation are Edoa and Js. However, 96 since C<sub>up</sub> is also a parameter that vary according to block orientation, it is considered in this study. 97 J<sub>s</sub>, E<sub>doa</sub> and C<sub>up</sub> do not report the same effect of joint orientation on rock mass erodibility. In order 98 to determine the parameter that best represents the effect of joint orientation on rock mass 99 erodibility, a method was used to assess the applicability and the effectiveness of each joint 100 orientation parameter to classify rock mass according to its vulnerability to erosion. This paper 101 covers the methodology used to define the parameter that best represents the effect of joint 102 orientation, presents a summary of the existing parameters, considers the results obtained and ends 103 with a discussion on the significance and validity of these results.

## 104 **2 Materials and Methods**

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106 The database used in this study was developed by Pells (2016). It includes the description, 107 geomechanical features and several geomechanical parameters of the rock mass in 24 dams located 108 in South Africa and Australia. Flow strength, flow direction and erosion damage levels are also 109 documented. For each dam, the spillway is separated into different sections depending on the 110 damage level. For each section, the following geomechanical parameters of joint sets are 111 described: opening, spacing, persistence, dip, orientation and roughness. The rock is also evaluated 112 with the Kirsten index factors: RQD,  $J_n$ ,  $J_r$ ,  $J_a$ ,  $J_s$  and  $M_s$ . In total, the database contains information 113 on a total of 114 sections spread across 24 dams. This information was reported in the form of 114 consultant reports, technical documents and observations made by geologists.

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The first step of the methodology is to develop the database.  $E_{doa}$  and  $J_s$  are determined by Pells (2016) for each section.  $C_{up}$  is determined according to the most critical joint orientation of each data set. Rock mass quality is assessed by the GSI<sub>chart</sub> rock resistance index, which is determined by Pells (2016). Stream power and damage levels are also defined by Pells (2016).

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The joint orientation parameters were compared together using the eGSI index, which combines a joint orientation parameter to a rock mass quality index. By varying the joint orientation parameter combined with the GSI<sub>chart</sub>, it is possible compare their effectiveness, given the assumption that eGSI is an effective method to determine erosion.

126 The methodology used to compare joint orientation parameters was inspired by Boumaiza et al. 127 (2019b). To combine similar data together, as it is a common practice in rock mechanics, each 128 parameter must first be divided into classes: joint orientation parameters, GSIchart, stream power 129 and damage level. For each GSI<sub>chart</sub> class, a graph of the mean stream power class in relation to the damage level class is produced. Joint orientation parameters are then plotted, according to the joint 130 131 orientation parameter class. It is then possible to compare the effectiveness of the joint orientation 132 parameter selected according to the damage level recorded. The methodology is summarized in 133 Figure 1.





### 136 2.1 Step 1 – Select a Joint Orientation Parameter

- 137 Step 1 consists of selecting one joint orientation parameter from those presented (Edoa, Js or Cup).
- 138 The following steps will be performed according to the selected parameter.
- 139 2.2 Step 2 Classify the Database According to GSI<sub>chart</sub>
- 140 The goal of this step is to combine data with similar rock quality index. GSI<sub>chart</sub> values are assigned

141 a class from 1 to 4, according to RMR classification (Bieniawski 1989). Classes are assigned

according to the separation shown in Figure A.1.1 in Appendix 1.

143 2.3 Step 3 – Classify the Joint Orientation Parameter Chosen for Each GSI Class

144 The goal of this step is to group data into different classes according to the value of the joint 145 orientation parameter previously chosen.

- 146 2.3.1 Js Parameter Classification
- 147 Classification of J<sub>s</sub> was done by Boumaiza et al. (2019b) by statistically distributing Pells (2016)
- 148 database. Classification of  $J_s$  is detailed in Table 1.
- 149 2.3.2 Edoa Parameter Classification
- 150 Classification of Edoa was done by Boumaiza et al. (2019b) by statistically distributing Pells (2016)
- 151 database. Classification of E<sub>doa</sub> is detailed in Table 2.
- 152 2.3.3 Cup Parameter Classification
- 153 Classification of Cup is presented in Table 3. It was done by statistically distributing Pells (2016)
- 154 database and based on the effect that this parameter has on erosion.
- 155 2.4 Step 4 Classify the Damage Level
- 156 The goal of this step is to assemble data with a similar damage level class from data with the same
- 157 GSI<sub>chart</sub> class and joint parameter class. The damage level classes were defined using Pells
- 158 classification (2016). The damage categories depend on the depth of the erosion and its general
- 159 extent. The classification proposed by Pells (2016) is presented in Table 4.

#### 160 2.5 Step 5 -Classify the Stream Power

The goal of this step is to group together all data with the same GSI<sub>chart</sub> class, the same joint orientation parameter class and the same damage class. Mean stream power is calculated using this data. A classification of stream power classes had already been proposed by Boumaiza et al. (2019b). However, this classification did not sufficiently account for the highest stream power, which is greater than 100kW/m<sup>2</sup>. This is why a modified classification is proposed, as shown in Table 5.

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168 When all the classes of the chosen joint orientation parameter have been evaluated, continue with 169 step 6. Otherwise, start over with a new joint orientation parameter class in step 3.

### 170 2.6 Step 6 – Draw the Damage Class Graph as a Function of the Mean Stream

#### 171 Power Class for the Chosen Joint Orientation Parameter

This step aims to visually compare the effect of each joint orientation parameter on rock mass erodibility. In total, four graphs will be created for each joint orientation parameter, with each graph representing one GSI<sub>chart</sub> class. The Y-axis represents the damage level class and the X-axis represents the mean stream power class for each class of the joint orientation parameter with the same damage class and GSI<sub>chart</sub> class.

If all GSI<sub>chart</sub> classes are considered for the same joint orientation parameter, continue with the
 next joint orientation parameter and repeat steps 1 to 6. Otherwise, choose a new GSI<sub>chart</sub> class and
 repeat steps 2 to 6.

#### 180 2.7 Step 7 – Interpret the Results

181 This step involves interpreting the results obtained and determining which joint orientation 182 parameters best represent the effect of joint orientation on rock mass erodibility.

### 183 **3 Theory**

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185 In the following section, the three joint orientation parameters introduced are presented in detail.

186 3.1 Presentation of the  $J_s$  Parameter

188 The J<sub>s</sub> parameter describes the effect of joint orientation on rock mass excavatability according to 189 the direction of excavation. Kirsten (1982) indicates it is easier to excavate a rock which contains 190 joints that dip in the same direction as the excavation rather than the opposite direction. In Kirsten's 191 study (1982), some assumptions are made to evaluate  $J_s$ : joint sets are considered orthogonal and 192 only the two closest joint sets are considered. Since Kirsten's index is used to assess rock mass 193 erodiblity rather than excavatability, in this article, J<sub>s</sub> is considered to be a joint orientation 194 parameter for rock mass erodibility. The joint spacing ratio (r) is defined by the ratio of  $S_{\Psi}$ , the 195 spacing between joints dipping in the opposite direction of ripping, and  $S_{\theta}$ , the spacing between 196 joints dipping in the direction of ripping (Figure A.1.2 in Appendix 1). The joint spacing ratio 197 presented in equation 3 is necessary to determine  $J_s$ .

198 
$$r = S_{\psi}/S_{\theta}$$

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200  $J_s$  values can be found from Table A.1.1 in Appendix 1. The first step in defining  $J_s$  is to define the 201 closest spaced joint set oriented perpendicular to the flow. Then, depending on the dip direction of 202 this joint set (whether it is in the same or opposite direction of the flow), the value of J<sub>s</sub> will be 203 found either in the upper section (for  $0^{\circ}$ ) or in the lower section (for  $180^{\circ}$ ) of Table A.1.1. With 204 the joint spacing ratio and the dip of this joint set, it is possible to find a value for J<sub>s</sub>. When joint 205 orientation has no effect on rock mass erodibility,  $J_s$  is equal to 1.  $J_s$  value will decrease below 1 if 206 joint orientation increases a rock's vulnerability to erosion. If joint orientation increases a rock's 207 resistance to erosion, the J<sub>s</sub> value will increase to above the value of 1.

(3)

## 208 3.2 Presentation of the Edoa Parameter

209 Pells (2016) added the Edoa parameter to the rock quality index GSI<sub>chart</sub> (Marinos and Hoek 2000) 210 to consider joint orientation and joint spacing effects on rock erosion, as GSIchart does not take 211 them into account. Edoa is based on the results of laboratory tests on a physical model, erosion, 212 geological and hydraulic data of dam sites, and on the theory of block uplift. Pells used the model 213 of the J<sub>s</sub> parameter to develop E<sub>doa</sub>. However, E<sub>doa</sub> is more precise, considering different parameters 214 such as flow types, channel slopes and various joint spacing ratios (r): 1: 1, 1: 2, 1: 4, 1: 8, 1:20. 215 For a horizontal channel and a bed-parallel flow, Edoa presents negative values, with a maximum 216 value of 0 and a minimum value of -18 (Figure 2). Therefore, joint orientation cannot have a 217 positive effect on the resistance to erosion of the rock, as for the J<sub>s</sub> parameter.



## 219 3.3 Presentation of the Cup Parameter

220 The Reinius (1986) and Montgomery (1984) laboratory experiments made it possible to obtain 221 different values of pressure acting on an instrumented block placed in a horizontal flow channel, 222 among an alignment of non-instrumented blocks (Figure 3a). The block was instrumented with 14 223 piezometers, which were used to measure the hydraulic head at different points (Figure 3b). The 224 block dimensions were 15 x 15 x 30 cm<sup>3</sup>. The Reinius (1986) and Montgomery (1984) studies 225 considers various flow rates, water loads above the block and block dips. As shown in Figure 3 (a) 226 and (b) the  $\beta$  angle is calculated from the vertical, whereas the dip is calculated from the horizontal. In order to compare the parameters, a 90° correction was made to transform ß values into dip 227 228 values.





$$231 c = \frac{h}{U^2/_{2g}} (4)$$

Where *h* represents the dynamic pressure (m), *U* represents the velocity of water (m s<sup>-1</sup>) and *g* represents the gravitational acceleration (m s<sup>-2</sup>).

- The dynamic pressure  $h_d$  is obtained from the water head recorded by the piezometer, from which
- the static pressure, the height of water from the piezometer, is subtracted (equation 5).

$$236 h_d = h_t - h_s (5)$$

- 237 Where  $h_t$  is the piezometer, i.e., the total water pressure, and  $h_s$  is the static pressure, the water load
- above the block. Cup was calculated with the mean value of the pressure coefficients at piezometers
- 239 5 and 8, when considering a block of thickness *b* (Figure 3 (b)).

## 240 **4 Results**

- 241
- 242 4.1 Classification of the GSI<sub>chart</sub> Index

According to the classes shown in Figure 2, GSI<sub>chart</sub> data distribution is presented in Figure 4 (a).
The Y-axis represents the total data for each GSI<sub>chart</sub> class.

245 4.2 Classification of the  $J_s$  Parameter

Data classification for the  $J_s$  parameter was performed according to the classes described in Table 1. Figure 4 (b) shows data distribution for each  $J_s$  class. Class 4 of  $J_s$  has significantly more data than the other classes. This class corresponds to a  $J_s$  equal to one, representing joint orientation having no impact on Kirsten's index, i.e., having no impact on rock resistance. The Y-axis represents total data for each  $J_s$  class.

251 4.3 Classification of the E<sub>doa</sub> Parameter

Data classification according to the  $E_{doa}$  parameter was done according to classes described in Table 2. Figure 4 (c) shows data distribution for each  $E_{doa}$  class. The  $E_{doa}$  of class 1 has far fewer data than the other classes. This class corresponds to a situation where  $E_{doa}$  has the least effect on rock mass resistance to erosion. The Y-axis represents total data for each  $E_{doa}$  class.

256 4.4 Classification of the C<sub>up</sub> Parameter

In order to assess a  $C_{up}$  value for each data set available, each joint set was analyzed according to its orientation in relation to the flow. Only joint sets not parallel to the flow were considered. A joint set must have a difference of orientation of at least 20° to be considered valid for this analysis. Then, according to Reinius (1986), the most critical joint set for each data set was chosen. The most critical joint set is the one with the highest  $C_{up}$  value. Figure 5 illustrates the distribution of data according to the  $C_{up}$  parameter and the orientation  $\beta$  (°) of the block. Only the most critical





264 two situations. The Y-axis represents total data for each Cup class.



-0,2

268 4.5 Effects of J<sub>s</sub> on Rock Mass Erosion

40

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ß angle (°)

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269 The effect of the  $J_s$  parameter are shown in Figure 6 (a-d). The Y-axis represents the mean stream 270 power class and X-axis represents the damage level class. The bubble size represent the amount of

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data for each bubble.  $J_s$  of class 1 (Js1) is defined by a joint orientation increasing the rock mass erodibility and  $J_s$  of class 5 (Js5) represents a joint orientation increasing rock mass resistance to erosion. If  $J_s$  correctly represents the effects of joint orientation on erosion, a high  $J_s$  class should produce less damage than a lower class. In addition, the relation between damage classes and the mean stream power class should be linearly increasing.



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278 For all GSI<sub>chart</sub> classes, the majority of the J<sub>s</sub> classes do not show the expected relation between 279 them. Indeed, the  $J_s$  of class 1 never show more damage than  $J_s$  of class 2. We can, however, 280 observe in Figure 6 (b) that the J<sub>s</sub> of class 2 show more damage than the J<sub>s</sub> of class 4, and for stream 281 power class above 3, more damage than  $J_s$  of class 3. Some correlation is observed for  $J_s$  of class 282 4 and 5. J<sub>s</sub> of class 4 shows indeed more erosion vulnerability than J<sub>s</sub> of class 5, supposed to show 283 erodibility resistance. We can also observe that the majority of the classes show the expected 284 relation between the damage level and mean stream power class, which is observed to be 285 increasing, as expected.

### 286 4.6 Effects of the Edoa Parameter

The effects of the  $E_{doa}$  parameter are shown in Figure 7 (a-d). If the  $E_{doa}$  parameter correctly represents the effects of joint orientation on erosion, a higher  $E_{doa}$  class should produce more damage than a lower class. In addition, the relation between damage classes and the mean stream power class should be linearly increasing.





### 307 4.7 Effects of the C<sub>up</sub> Parameter

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309 The effects of the  $C_{up}$  parameter are shown in Figure 8 (a-d). The Y-axis represents the damage 310 level and the X-axis represents the mean stream power class. If the  $C_{up}$  parameter correctly 311 represents the effects of joint orientation on erosion, higher classes should show more damage than 312 lower  $C_{up}$  classes. In addition, the relation between damage classes and mean stream power should 313 be linearly increasing.



For the GSI<sub>chart</sub> classes 1 and 2, i.e., for good-quality rock masses, the results show an acceptable classification of the  $C_{up}$  parameter. It is generally observed that higher  $C_{up}$  classes generate higher damage levels, for high stream power classes. However, for lower-quality rock mass, for GSI classes 3 and 4, correlation between damage and  $C_{up}$  classes is no longer observed. It is also noted that the relation between stream power and damage is generally increasing.

#### 321 4.8 Comparison of Results

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In general, the parameter classification results improve as the mean stream power classes increase. 323 324 For the Edoa parameter, its classification also improves when the GSI<sub>chart</sub> increases. An increase in 325 the GSI<sub>chart</sub> classes, according to the classification proposed in this article, occurs when the quality 326 of the rock mass decreases. Thus, joint orientation would have a greater impact on erodibility for 327 a lower quality rock subjected to high hydraulic pressures. However, this is counterintuitive since 328 in a very low-quality rock mass i.e., for a GSI<sub>chart</sub> below 40, corresponding to a GSI<sub>chart</sub> of class 4, 329 the rock is crushed and joint orientation is not easily distinguished. Joint orientation is best 330 distinguished when rock mass quality is high. Results show correlation between joint orientation 331 and damage level when GSI<sub>chart</sub> is of class 1 and class 2, for high stream power, only for the C<sub>up</sub> 332 parameter. No joint orientation parameter presents an accurate relationship between damage level 333 and joint orientation effect for a good quality rock mass. For all GSI<sub>chart</sub> classes, solely J<sub>s</sub> classes 4 334 and 5 show correlation with damage classes. These classes describe joint orientation that would 335 either have no effect or have a positive effect on rock resistance to erosion.

## 336 **5 Discussion**

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There are several methods used to classify the effect of joint orientation in a given rock mass on its erodibility. These methods offer different classifications, hence the need to evaluate these methods and compare their accuracy.

341 The errors in the classification of the  $J_s$  parameter developed by Kirsten (1982) can be explained 342 by four factors, namely the assumption that joints are orthogonal to each other, the fact that  $J_s$ 343 parameter has been developed to characterize the excavatability of the rock rather than erodibility, 344 the fact that this parameter was developed only theoretically, and the poor amount of data for all 345  $J_s$  classes except for  $J_s$  of class 4 (Figure 6). Boumaiza et al (2019a) show that the approximation 346 of orthogonal joints leads to significant errors when calculating Kirsten's index when the joint sets 347 considered are in fact non-orthogonal. J<sub>s</sub> is obtained with the direction of the closer spaced joint 348 set and the apparent dip of the closer spaced joint set with relation to the direction of the flow. 349 However, the apparent dip is calculated considering the two joint sets as being orthogonal. If they 350 are in fact non-orthogonal, this apparent dip will change along with the J<sub>s</sub> value. In addition, 351 Kirsten's index, considering the  $J_s$  parameter, was developed to characterize the excavatability of 352 a rock mass. The force considered when excavating a rock mass is that of a bulldozer, where the 353 force applied by the excavator acts only at a specific point and is directed at a specific angle. The 354 study of erodibility by hydraulic power involves a hydraulic force acting on all the faces of the 355 blocks of the rock mass, particularly with an uplift force that acts under the block. Since this 356 parameter was developed wholly theoretically, some inconsistencies may have appeared, like the 357 fact that joint orientation can have a positive effect on rock resistance to erosion. Since this 358 methodology was applied on a limited data set, data was unequally distributed through  $J_s$  classes, 359 which resulted in too few data for four out of five of J<sub>s</sub> classes.

360 The  $E_{doa}$  parameter was developed by Pells (2016). It is based on the J<sub>s</sub> parameter, the results of 361 tests on a scale model, real erosion situations that have taken place in dams and on the kinematics 362 of block uplift. Pells (2016) physical model has flaws regarding the analysis of joint orientation 363 effects. Indeed, only one block is modelled, not allowing to measure the effect that joint orientation 364 could have on a series of blocks and joints. Multiple blocks and joints surrounding an instrumented 365 block could have an incidence on the uplift pressure measured on that block. Also, only a few orientations were tested, that is 0°, 22° and 45° horizontally and 0°, 11° and 22° vertically. 366 Horizontal and vertical orientations correspond to direction and to dip direction, respectively. The 367 368 Edoa parameter also considers orthogonal joints, which presents the same problem as for the Js 369 parameter. Pells (2016) built his database with erosion and dam data coming from different 370 sources. Since Pells (2016) did not collect all data himself, some errors or different interpretations 371 coming from different sources could have been included, especially regarding more qualitative parameters, such as GSI<sub>chart</sub>. An analysis of the results demonstrates that the E<sub>doa</sub> parameter offers 372 373 the best classification of the joint orientation effect on rock mass erodibility. However, the 374 database used to build the  $E_{doa}$  parameter was used for this study, which could introduce bias into 375 the results of Edoa.

The C<sub>up</sub> parameter of Reinius (1986) is exclusively based on the pressure results obtained from a scale model. The advantage of the C<sub>up</sub> parameter is that it is the only parameter based exclusively on laboratory tests. It is, therefore, possible to precisely understand its origin. The physical model used consisted of an alignment of blocks, one of which was instrumented with piezometers. The uplift pressure parameter is not calculated directly from the pressures measured under the block, but is calculated as the mean value of the pressure coefficients of piezometers 5 and 8. Moreover, pressure variation was not measured as a variable dependent of time, more as a fixed value for 383 each model setup. The  $C_{up}$  parameter yields opposite results to the  $E_{doa}$  parameter, with a better 384 correlation for high GSI classes. In Reinius (1986) experiments to development the Cup parameter, 385 the rock mass was modelled with concrete blocks. These blocks thus represent a rock mass of high 386 quality, which corresponds with GSI classes 1 and 2. Therefore, the Cup parameter may only be 387 applicable to good GSI classes. Unlike Edoa and Js, Cup does not consider joint spacing. The joint 388 spacing considered for the  $C_{up}$  parameter is the one used in Reinius (1986) experiments, which is 389 has a fixed value for all tests. According to Edoa and Js, joint spacing does not radically change the 390 impact of joint orientation, but it does change the value slightly. Additionally, joint orientation 391 testing by Reinius (1986) was limited. The block was always placed vertically in the model, with varying dips from the vertical of  $0^{\circ}$ ,  $9^{\circ}$ ,  $17.5^{\circ}$ , and  $33.5^{\circ}$  in the opposite direction of the flow and 392 393 varying dips from the vertical of 2.9° and 18° in the same direction of the flow. These tests allowed 394 Reinius (1986) to extrapolate his results to all cases. However, the Cup parameter would need to 395 be further developed in order to be applied to all cases of joint orientation.

396 This study reveals some probable inconsistencies with the GSI<sub>chart</sub> determination in relation to joint 397 orientation. Considering the joint orientation parameters analyzed, results show that joint 398 orientation has much less of an effect on good-quality rock mass than on lower-quality rock mass. 399 However, joint orientation is much more distinguishable on a good quality rock mass. GSI<sub>chart</sub> is 400 determined using block structure and joint condition. When considering GSI<sub>chart</sub>, the same GSI<sub>chart</sub> 401 value is determined for a "very blocky" rock mass with a good joint condition (rough and not 402 altered) as for a "massive" rock mass with a medium joint condition (smooth and altered), which 403 could explain the issue regarding the effect of joint orientation. To effectively compare the effect 404 of joint orientation, rock mass should be classified according to the same class of structure and the 405 same class of joint condition. Moreover, in our analysis with GSI<sub>chart</sub>, joint orientation and stream 406 power does not consider all parameters as having an effect on rock mass erosion, such as NPES 407 and joint opening. Boumaiza et al (2021) revealed a classification of the most relevant parameters 408 to study rock mass hydraulic erodibility. From the most to the least important, his study obtained 409 the following classification: joint condition ( $K_d$ ), nature of the potentially erodible surface (NPES), 410 block volume (V<sub>b</sub>), joint opening (J<sub>o</sub>), joint orientation (E<sub>doa</sub>) and rock mass deformation module 411 (Erm). Our study does not consider NPES, which includes the block's protrusion, nor does it 412 consider joint opening or rock deformation module. It would be interesting to include NPES and 413 J<sub>o</sub> in an index of rock quality, as they have more importance for rock mass hydraulic erosion than

414 joint orientation. Furthermore, joint condition and block volume have the same weight when 415 determining GSI<sub>chart</sub>, when, according to Boumaiza et al (2021), joint condition is the most 416 important parameter for rock mass hydraulic erodibility with block volume following in third 417 position. Therefore, when studying rock mass hydraulic erodibility, a rock quality index that takes 418 this classification into account should be considered.

## 419 **6** Conclusion

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421 Hydroelectric facilities require the excavation of an emergency spillway in the bedrock, which 422 exposes the rock mass to hydraulic erosion. Joint orientation is known as a relevant geomechanical 423 parameter to evaluate rock mass erodibility. The method used in this article to evaluate the 424 parameters describing the effects of joint orientation on rock mass erodibility is based on the one 425 used by Boumaiza et al (2019b). The results show that the  $E_{doa}$  parameter is the joint orientation 426 parameter with a classification that is closest to expectations, based on the GSI<sub>chart</sub> index of rock 427 quality, mean stream power and damage level. The Edoa parameter shows good correlation when 428 the rock is of medium to low quality. Good results for the E<sub>doa</sub> parameter may be influenced due 429 to the database used to determine  $E_{doa}$  being the same used to analyze its accuracy in our study. 430 This leaves room for potential bias. Regarding the  $J_s$  parameter, the classification obtained is not 431 representative of the damage level. However, little data is available for  $J_s$  classes 1, 2, 3 and 5, 432 since the majority of data falls under J<sub>s</sub> class 4. Regarding the C<sub>up</sub> parameter, for high stream 433 power, results generally show good correlation for GSI<sub>chart</sub> classes 1 and 2. Unlike the E<sub>doa</sub> and J<sub>s</sub> 434 parameters, C<sub>up</sub> does not consider spacing ratio of joint sets, which could help increase the 435 accuracy of the results for this parameter.

436 Good-quality rock masses can have a variety of structures, from massive to very blocky, with a 437 joint condition ranging from rough and non-altered to smooth and altered. In this study, some 438 parameters that have a significant impact on rock mass erodibility were disregarded, such as NPES, 439  $J_o$  and  $E_{rm}$ . Development of a rock quality index that includes these parameters and considers their 440 relative importance would be useful in order to correctly analyze joint orientation parameters and 441 rock mass erodibility.

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448

## 449 8 Authors contribution

450 Marie-Hélène Wisse: Conceptualization, Validation, Visualization, Formal analysis,

- 451 Investigation, Methodology, Writing Original Draft, Writing Review and Editing
- 452 Ali Saeidi: Conceptualization, Methodology, Resources, Writing Review and Editing,
- 453 Supervision, Project administration, Funding acquisition, Validation

454 Marco Quirion: Writing – Review and Editing, Supervision, Funding acquisition

- 455
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# 462 **10 Data Availability Statement**

463 All data generated or analysed during this study are included in this published article: (Pells464 2016).

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Class	$\mathbf{J}_{\mathbf{s}}$	Description
1	0.4-0.6	Highly vulnerable to erosion
2	0.6-0.8	Very vulnerable to erosion
3	0.8-<1	Moderately vulnerable to erosion
4	1	Less vulnerable to erosion
5	>1	Minimally vulnerable to erosion

Table 1. Js proposed classification (Boumaiza et al. 2019b)

Table 2. *Edoa proposed classification (Boumaiza et al. 2019b)* 

Class	Edoa	Description
1	0 to -5	Minimally vulnerable to erosion
2	-5 to -10	Less vulnerable to erosion
3	-10 to -15	Moderately vulnerable to erosion
4	-15 to -25	Highly vulnerable to erosion

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Table 3. Cup proposed classification

Class	Cup	Description
1	<0	Minimally vulnerable to erosion
2	0-0.1	Less vulnerable to erosion
3	0.1-0.4	Moderately vulnerable to erosion
4	0.4-0.5	Very vulnerable to erosion
5	>0.5	Highly vulnerable to erosion

Table 4. Proposed classification of damage levels (Pells 2016)

Class	Scour depth (m)	General extent m <sup>3</sup> 100m <sup>-2</sup>	Damage description
1	< 0.3	<10	Negligible
2	0.3 - 1	10 - 30	Minor
3	1 - 2	30 - 100	Moderate
4	2 - 7	100 - 350	Large
5	>7	>350	Extensive

Table 5. Proposed stream power classification. Modified from Boumaiza et al (2019b)

Class	Stream Power (∏uD, kW m <sup>-2</sup> )	Description
1	< 2.5	Very Low
2	2.5 - 10	Low
3	10 - 25	Moderate

4	25 - 50	High
5	50 - 100	Very High
6	> 100	Extreme

528 Table in the Appendix

529 Table A.1.1. *Relative ground structure number (Js) proposed values rebuilt from Kirsten (1982)* 

Dip direction of	Dip angle of closer	Ratio of joint spacing (r)				
closer spaced joint set	spaced joint set (°)	1:1	1:2	1:4	1:8	
(°)	· ·					
180/0	90	1	1	1	1	
0	85	0.72	0.67	0.62	0.56	
0	80	0.63	0.57	0.50	0.45	
0	70	0.52	0.45	0.41	0.38	
0	60	0.49	0.44	0.41	0.37	
0	50	0.49	0.46	0.43	0.40	
0	40	0.53	0.49	0.46	0.44	
0	30	0.63	0.59	0.55	0.53	
0	20	0.84	0.77	0.71	0.68	
0	10	1.22	1.10	0.99	0.93	
0	5	1.33	1.20	1.09	1.03	
0/180	0	1	1	1	1	
180	5	0.72	0.81	0.86	0.90	
180	10	0.63	0.70	0.76	0.81	
180	20	0.52	0.57	0.63	0.67	
180	30	0.49	0.53	0.57	0.59	
180	40	0.49	0.52	0.54	0.56	
180	50	0.53	0.56	0.58	0.60	
180	60	0.63	0.67	0.71	0.73	
180	70	0.84	0.91	0.97	1.01	
180	80	1.22	1.32	1.40	1.46	
180	85	1.33	1.39	1.45	1.50	
180	90	1	1	1	1	

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532 Fig. 1: Methodology for comparing joint orientation parameters

533 Fig. 2: E<sub>doa</sub> values for a horizontal channel and a bed-parallel flow (Pells 2016)

*Fig. 3: (a) Experimental setup. Reworked from Reinius (1986) (b)* Simulated fracture between piezometers 5 and 8.
Dynamic pressures are also shown. Reworked from Reinius (1986)

Fig. 4: (a) Distribution of data by GSI<sub>chart</sub> class (b)Distribution of J<sub>s</sub> classes of Pells (2016) case studies data (c)
 Distribution of E<sub>doa</sub> classes of Pells (2016) case studies data (d) Distribution of data according to Reinius' study.
 Modified from Reinius (1986)

539 Fig. 5: Distribution of Pells' (2016) data by  $C_{up}$  classification

540 Fig. 6: Results of the effects of the  $J_s$  parameter on the rock's vulnerability to erosion. Lines represent the linear 541 approximation of data distribution (a) GSI<sub>chart</sub> class 1 (b) GSI<sub>chart</sub> class 2 (c) GSI<sub>chart</sub> class 3 (d) GSI<sub>chart</sub> class 4

542 Fig. 7: Results of the effects of the  $E_{doa}$  parameter on the rock's vulnerability to erosion. Lines represent the linear 543 approximation of data distribution (a) GSI<sub>chart</sub> class 1 (b) GSI<sub>chart</sub> class 2 (c) GSI<sub>chart</sub> class 3 (d) GSI<sub>chart</sub> class 4

- 544 545 Fig. 8 Results of the effects of the  $C_{up}$  parameter on the rock's vulnerability to erosion. Lines represent the linear approximation of data distribution (a)  $GSI_{chart}$  class 1 (b)  $GSI_{chart}$  class 2 (c)  $GSI_{chart}$  class 3 (d)  $GSI_{chart}$  class 4
- 546 547 548 549 550
- Figures in the Appendix
- Fig. A.1.1: GSI determination chart and class separation modified from Marinos and Hoek (2000)
- 551 Fig. A.1.2: Sketch of fractured rock mass (Kirsten 1982)
- 552

GEOLOG JOINTED From the conditio the avera- be too p to 37 is r GSI = 35. apply to Where w present i with resp will dom The shea that are of chang reduced with rock categorie for wet c with by o	ICAL STRENGTH INDEX FOR PROCKS Elithology, structure and surface age value of GSI. Do not try to recise. Quoting a range from 33 more realistic than stating that Note that the table does not structurally controlled failures. reak planar structural planes are in an unfavourable orientation bect to the excavation face, these inate the rock mass behaviour. ar strength of surfaces in rocks prone to deterioration as a result jes in moisture content will be if water is present. When working ks in the fair to very poor es, a shift to the right may be made conditions. Water pressure is dealt effective stress analysis STRUCTURE	SURFACE CONDITIONS	VERY GOOD Cough, fresh, unweathered surfaces	5 6 6 7 6 7 8 8 8 8 8 9 8 9 8 9 8 9 8 9 9 9 9 9 9		것 POOR Slickensided, highly weathered surfaces with compact 미 coating or fillings of angular fragments	VERY POOR Slickensided, highly weathered surfaces with soft clay coatings or fillings
	INTACT OR MASSIVE- Intact rock speciments or massive in- situ rock with few widely spaced discontinuities		90 <b>1</b> 80	****		N/A	N/A
	BLOCKY - Well interlocked un- disturbed rock mass consisting of cubical blocks formed by three intersecting discontinuity sets	OCK PIECES	***/	70 6Ô			
	VERY BLOCKY - Interlocked, partially disturbed mass with multi-faceted angular blocks formed by 4 or more joint sets	LOCKING OF R		*//	50	*	
	BLOCKY/DISTURBED/SEAMY - Folded with angular blocks formed by many intersecting discontinuity sets. Persistence of bedding planes or schistosity	REASING INTER			40	<b>^</b>	
	DISINTEGRATED - Poorly inter- locked, heavily broken rock mass with mixture of angular and rounded rock pieces	- B I		a a a a a a a a a a a a a a a a a a a		20	//
	LAMINATED/SHEARED - Lack of blockiness due to close spacing of the weak schistosity or shear plan	nes	N/A	N/A	$\square$		10



Dip direction of	Dip angle of closer	Ratio of joint spacing (r)			
closer spaced joint set	spaced joint set (°)	1:1	1:2	1:4	1:8
(°) <b>*</b>	1 5 ()				
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180	85	1.33	1.39	1.45	1.50
180	90	1	1	1	1